PREDICTING THE BEHAVIOUR OF FLUSH END PLATE CONNECTIONS USING STRAIN GAUGES

NORHASHILA SHUHADA BTE TUAH

UNIVERSITI TEKNOLOGI MALAYSIA

UNIVERSITI TEKNOLOGI MALAYSIA

DECLARATION OF THESIS / PROJECT REPORT AND COPYRIGHT			
Author's full name	: <u>NORHASHILA SH</u>	IUHADA BINTI TUAH	
Date of birth	: <u>18/7/1985</u>	<u> </u>	
Title		E BEHAVIOUR OF FLUSH END PLATE SING STRAIN GAUGES	
Academic Session	: <u>2009/2010</u>		
I declare that this thesi	s is classified as :		
	TAL (Contains c Secret Act	onfidential information under the Official 1972)*	
		estricted information as specified by the n where research was done)*	
	ESS I agree that access(full	t my thesis to be published as online open text)	
l acknowledged tha	t Universiti Teknologi /	Malaysia reserves the right as follows:	
1. The thesis is th	ne property of Univer	siti Teknologi Malaysia.	
-	 The Library of Universiti Teknologi Malaysia has the right to make copies for the purpose of research only. 		
3. The Library here exchange.	as the right to make	copies of the thesis for academic	
		Certified by :	
(SIGI	NATURE)	SIGNATURE OF SUPERVISOR	
<u>850718</u>	8-01-5852	DR ARIZU SULAIMAN	
(NEW IC NO.	/PASSPORT NO.)	NAME OF SUPERVISOR	
Date: 20 NC	OVEMBER 2009	Date: 20 NOVEMBER 2009	

NOTES: * If the thesis is CONFIDENTAL or RESTRICTED, please attach with the letter from the organization with period and reasons for confidentiality or restriction.

"I hereby declare that I have read this project report and in my opinion this project report is sufficient in terms of scope and quality for the partial fulfillment of the requirements for the award of the degree of Master of Engineering (Civil - Structure)"

Signature	:
Supervisor	: DR ARIZU BIN SULAIMAN
Date	: 20 NOVEMBER 2009

PREDICTING THE BEHAVIOUR OF FLUSH END PLATE CONNECTIONS USING STRAIN GAUGES

NORHASHILA SHUHADA BTE TUAH

A project report submitted in partial fulfillment of the requirements for the award of the degree of Master of Engineering (Civil - Structure)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > November 2009

I declare that this project report entitled "*Predicting the Behaviour of Flush End Plate Connections using Strain Gauges* "is the result of my own research except as cited in the references. The project report has not been accepted for any degree and is not concurrently submitted in candidature of any other degree.

Signature	:
Name	: NORHASHILA SHUHADA BTE TUAH
Date	: 20 NOVEMBER 2009

TO MY BELOVED

FATHER TUAH BIN NGAH MOHD

MOTHER Pn ROSNI BT ISMAIL

SISTER NURUL QURATU AIN NURSYAIDATUL NATRAH

MOHD REDZUAN HUSSIN

ACKNOWLEDGEMENT

In preparing this study, I was in contact with many people, researchers, academicians, and practitioners. They have contributed towards my understanding and thoughts. In particular, I wish to express my sincere appreciation to my main thesis supervisor, Dr. Arizu Bin Sulaiman, for encouragement, guidance, advices, motivation, critics and friendship. Without his continued support and interest, this study would not have been the same as presented here.

Next, I would like to thank all my friends who have helped me throughout my postgraduate study. Special thanks go towards Miss Fatimahtuzahrah Binti Mohd Mahir, who has extended their hands in helping me in completing this research. My sincere appreciation also extends to all my colleagues and others who have provided assistance at various occasions. Their views and tips are useful indeed. Unfortunately, it is not possible to list all of them in this limited space. I am grateful to all my family members.

ABSTRACT

Connections are commonly designed as simple or rigid although the actual behavior is known to fall between these two extreme cases. This study presents the results of an analysis of strain gauge data from the already conducted experimental work on flush end plate connections for trapezoid web profiled (TWP) steel section. The experimental work on these partial strength connections has been done by Md Tahir, M. et al. in 2006. The objective of this study is to manipulate the strain values in predicting the behavior of flush end connections from isolated tests in term of the capacity. The flush endplate connections used in this study are partial strength with TWP steel section as beam and hot rolled U-C section as column. The relationships between moment and rotation of each connection were obtained, and performances were evaluated in terms of the stiffness, moment capacity and ductility. The results show that the pattern and trend of moment rotation curves from the strain gauge analysis are in good agreement with the ones obtained using inclinometers. Therefore, it can be concluded that the strain gauge data can be used to predict the behaviour of the flush end plate connections. Furthermore, the results also show that the moment capacity of the connection increases as the size of beam increases. The small percentage of different between the predicted values and to exact values obtained using the inclinometer shows that strain gauges can be used as the alternative data collecting devices in predicting the behaviour of the flush end plate connections

ABSTRAK

Rekabentuk sambungan kebiasaan direka sebagai sambungan mudah dan sambungan tegar tetapi kelakuan sambungan yang sebenar adalah separuh tegar. Kajian ini memberi keputusan daripada analisis data ujikaji tolok terikan yang telah dijalankan sebelum ini keatas plat hujung sedatar menggunakan rasuk berprofil trapezoid. Ujikaji keatas sambungan separuh tegar ini telah dijalankan oleh Md Tahir, M. et al. pada 2006. Objektif kajian ini adalah memanipulasikan nilai terikan untuk meramalkan kelakuan sambungan keluli jenis plat hujung sedatar dalam bentuk kapasiti. Sambungan keluli jenis plat hujung sedatar telah diguna sebagai sambungan separuh tegar, di samping rasuk berprofil trapezoid dan tiang berbentuk U-C. Hubungan antara momen dan putaran untuk setiap sambungan boleh diperolehi dan prestasi sambungan tersebut dihasilkan dalam bentuk kekukuhan, momen keupayaan dan kemuluran. Daripada keputusan yang diperolehi, ia menunjukkan bentuk lengkungan momen putaran daripada analisis tolok terikan mempunyai keserataan yang baik dengan keputusan yang dihasilkan melalui ujikaji inclinometer. Kesimpulannya, data daripata tolok terikan boleh digunakan untuk menganggarkan kelakuan sambungan plat hujung sedatar. Selain itu, keputusan yang diperolehi juga menunjukkan nilai moment keupayaan bertambah bila saiz rasuk bertambah. Peratus perbezaan yang kecil antara nilai jangkaan dan nilai yang sebenar dari *inclinometer* menunjukkan tolok terikan boleh digunakan sebagai salah satu alat dalam menjangka kelakuan sambungan plat hujung sedatar.

TABLE OF CONTENT

CHAPTER	TITLE	PAGE
	DECLARATION	ii
	DEDICATION	iii
	ACKNOWLEDGEMENT	iv
	ABSTRACT	V
	ABSTRAK	vi
	TABLE OF CONTENT	vii
	LIST OF TABLE	Х
	LIST OF FIGURE	xi
1	INTRODUCTION	
	1.1 General	1
	1.2 Problem Statement	2
	1.3 Objective	3
	1.4 Scope of Studies	4
2	LITERATURE REVIEW	
	2.1 Beam to Column Connection	5
	2.2 Types of Connection	8
	2.2.1 Simple Connection	9
	2.2.2 Rigid Connection	11
	2.2.3 Semi-Rigid Connection	12

2.3	Characteristic of Connection	15
	2.3.1 Moment Resistance	16
	2.3.2 Rotational Stiffness	17
	2.3.3 Rotational Capacity	17
2.4	Moment Rotation Relationship	18
2.5	End Plate Connection	25
2.6	Flush End Plate	27
2.7	Trapezoid Web Profile	28
2.8	Strain Gauge Data	30
	2.8.1 What is Strain Gauge	31
	2.8.2 Operation of Strain Gauge	34
	2.8.3 Strain Gauge Application	37
	2.8.4 Function of Strain Gauge	37
	2.8.5 Strain Gauge Selection	38
	2.8.6 Strain Gauge Rosettes	40
	2.8.7 Types of Strain Gauge Rosettes	41
	2.8.8 Data from Strain Gauge	43
2.9	Experimental Work	44
	2.9.1 Experimental Setup	44
	2.9.2 Specimen	47
	2.9.3 Instrumentation System	52

3 METHODOLOGY

3.1 Introduction	54
3.2 Methodology Flow Chart	55
3.3 Location of Strain Gauge	57
3.4 Stress Strain Relationship	58
3.5 Load versus Strain Curve	61
3.6 Important Parameter	62
3.7 Moment Rotation Relationship	63
3.8 Load versus Deflection	64

4

5

RESULTS AND DISCUSSION

4.1 Introduction		66
4.2 Significant Loc	ation of Strain Gauge	67
4.3 Result from Pre	evious Experiment	72
4.4 Result of Analy	tical Investigation	77
4.4.1.Moment	Rotation Relationship	77
4.4.1.1.	Moment	78
4.4.1.2.	Deflection	79
4.4.1.3.	Rotation	79
4.4.1.4.	Moment Rotation Curve	85
4.4.2.Load vers	sus Deflection	88
4.5 Previous Result	of Finite Element	91
4.6 Result Compari	son	92
4.6.1.Comparis	ons with Beam and Bolt of Specimen	93
4.6.2.Comparis	on with Inclinometer	95
4.6.3.Comparis	on of Moment Resistance, M_R and	98
Ultimate	Load, P _U	
4.6.4.Comparis	on with Finite Element	101
4.6.5.Comparis	on at All Specimen	102
CONCLUSIONS A	ND RECOMMENDATIONS	
5.1 Conclusions		104

J.1 COliciusions	104
5.2 Recommendations	106

REFERENCES	107

LIST OF TABLES

TABLE NO	TITLE	PAGE
2.1	Common Configurations	35
2.2	Strain rosettes and principle stresses	43
2.3	Details of specimens for the isolated test	50
3.1	Test result for the tensile test	60
4.1	Significant location strain gauge for specimen N1	67
4.2	Significant location strain gauge for specimen N2	68
4.3	Significant location strain gauge for specimen N3	70
4.4	Significant location strain gauge for specimen N9	71
4.5	Test result based on the moment versus rotation plots	76
4.6	Result from the moment rotation curve	88
4.7	Test result based on the load versus deflection plots	91
4.8	Comparison moment resistance and moment ultimate	99
4.9	Comparison of ultimate load	100
4.10	The comparison of finite element with result from strain	102
	gauge	

LIST OF FIGURE

FIGURE	TITLE	PAGE
NO		
2.1	Beam to column connection, A) fully bolted, B) Finplate	6
	C) Endplate, D) Flush EndplaStrain rosettes and principle	
	stresses	
2.2	Illustrates the 6 possible failure modes for a fin plate	7
2.3	Moment-Rotation of connection	8
2.4	Simple connections	9
2.5	Simple beam to column connection,1) fin plates, 2) angle	10
	cleats, 3)flexible endplate welded, 4)bolted with angle	
	cleats, 5) tube, 6) stiffness plate	
2.6	Rigid connections	12
2.7	Semi-rigid connections	13
2.8	Type of semi rigid connections	14
2.9	Classification by strength	16
2.10	Classification by rigidity	17
2.11	Classification by ductility	18
2.12	Non-dimensional connections classification diagram	19
2.13	Moment rotation diagram (М-фсиrves)	20
2.14	Moment rotation curve for partial strength	21
2.15	Comparison of moment rotation relationship between	22
2.16	Moment Rotation curve for end plate connection	23
2.17	Schematization of rotational stiffness for the frame	23

analysis

2.18	Various forms of M-фcurves	24
2.19	Possible idealisations for M-dcurves	25
2.20	Types of end plate connection	26
2.21	Force applied to end plate connection	27
2.22	Flush end plate connection	28
2.23	Trapezoidal web profile	29
2.24	Example of strain gauge	30
2.25	Initially a segment of metal wire with a cross-sectional	32
	area, A_o , and length, l_{o} is illustrated.	
2.26	Geometry of strain gauges	33
2.27	An unbalanced Wheatstone bridge	35
2.28	Quarter Bridge circuit	36
2.29	Half Bridge circuit	36
2.30	Full bridge circuit	36
2.31	Rectangular Rosettes	42
2.32	Delta rosettes	42
2.33	Arrangement of isolated experiment	45
2.34	Arrangement of sub-assemblage experiment	46
2.35	a) Compressive axial loading frame, b) Hydraulic jack, c)	46
	Central load cell	
2.36	Specimen N1	47
2.37	Specimen N2	47
2.38	Specimen N3	48
2.39	Specimen N4	48
2.40	Specimen N9	49
2.41	Geometry configuration of partial strength connection	51
2.42	Locations of the strain gauges for the flush end plate	53
3.1	Flow chart of methodology	56
3.2	Illustration of location strain gauge	57

3.3	Stress strain relationship diagram; 1 - Ultimate Strength, 2	59
	- Yield Strength (elastic limit), 3 - Rupture, 4 - Strain	• •
	hardening region, 5 - Necking region, A: Apparent stress	
	(F/A_0) , B: Actual stress (F/A)	
3.4	Typical stress strain relationship	59
3.5	Load- strain curve at flange	61
3.6	Area of significant and not significant strain value	62
3.7	Moment Rotation Curve	63
3.8	Example of moment Rotation Curve	64
3.9	Load versus deflection	65
4.1	Moment versus rotation for specimen N1	72
4.2	Moment versus rotation for specimen N2	73
4.3	Moment versus rotation for specimen N3	73
4.4	Moment versus rotation for specimen N4	74
4.5	Moment versus rotation for specimen N9	74
4.6	Applied load P and the moment arm	78
4.7	Calculation of rotation	80
4.8	Illustration of location strain gauge at beam	81
4.9	Rotation of the connection	82
4.10	Rotation	82
4.11	Location of strain gauge at bolt	83
4.12	Rotation of connection	84
4.13	Illustration of rotation at bolt	84
4.14	Moment rotation curve for specimen N1	86
4.15	Moment rotation curve for specimen N2	86
4.16	Moment rotation curve for specimen N3	87
4.17	Moment rotation curve for specimen N9	87
4.18	Load versus Deflection for specimen N1	89
4.19	Load versus Deflection for specimen N2	89
4.20	Load versus Deflection for specimen N3	90
4.21	Load versus Deflection for specimen N9	90

4.22	Moment Rotation curve for specimen N2 using finite	92
	element analysis.	
4.23	Comparison beam and bolt for specimen N1	94
4.24	Comparison beam and bolt for specimen N2	94
4.25	Comparison beam and bolt for specimen N9	95
4.26	Moment-rotation curve for experiment and strain gauge	96
	analysis at specimen N1	
4.27	Moment-rotation curve for experiment and strain gauge	97
	analysis at specimen N2	
4.28	Moment-rotation curve for experiment and strain gauge	97
	analysis at specimen N3	
4.29	Moment-rotation curve for experiment and strain gauge	98
	analysis at specimen N9	
4.30	Comparison with finite element	101
4.31	Moment-Rotation curve for all specimen	103

CHAPTER 1

INTRODUCTION

1.1 General

Steel as a structural material has several desirable qualities such as high strength, high stiffness and high ductility. It is the strongest, most versatile and economical material available to the construction industry and its high ductility enables it to withstand large deformations at high stress levels without rupture. Strength is the ability of a material to resist stresses; stiffness is the ability of a material to resist deformation; toughness is the ability of a material to absorb energy before failure meanwhile ductility is the ability of a material to undergo large plastic deformation before failure [1].

Steel structures are built using many components, such as tension members, compression members, bending members, combined force members and connections [2]. Amongst the said components, connections are the most critical. Most steel structure failures are caused by inadequate and poorly designed connections, while failure due to main structural members is rare. Poorly or wrongly design connections can lead to a failure and can affect the stresses of the main structural members. They affect the overall behavior of the structure.

Steel frame, the connection between the beam and column is either assumed as pinned, where only nominal moment from the beam is transferred to the column or rigid connection where full continuity of moment transfer exists [3]. However, a majority of the actual connections show partial strength behaviour but the ability to predict the behaviour of this connection is still limited. The limited is because to understanding the real behaviour of this connection compares another connection. Same with another connection, semi rigid connection design requires the moment rotation relationships, which include the moment resistance and rotational stiffness to be established prior to its usage in design. The purpose of using Trapezoid Web Profile sections is to take advantage of the benefits offered by the section in general [4].

1.2 Problem Statement

In steel structure, connection is a one a most importance part in structure strength. Mostly connection designed in past are assumed either perfectly pinned or fully rigid connection although the actual behavior is known to fall between these two extreme connections. While the types connection of flush end plate is a semi rigid connection. A semi rigid flush end plate is representing various complexity and undefined problems with many parameters affecting its behavior and structural capacity. The behaviour of semi rigid is not very familiar like other types of connection. So to understand more about real behaviour of the semi rigid connection, one laboratory testing can be conducted. The experiment is strain gauge analysis.

Commonly the behavior of connections can be show in term of moment rotation curve and basically can be obtained from the experiment of inclinometer. When one situation did not have inclinometer equipment or data but only have strain gauge, so at this situation the analysis must be done from the data of strain gauge to predict the behaviour of this connection.

1.3 Objective

The objective of this research is:

- 1.3.1 To analyse the strain gauge data of flush end plate connection.
- 1.3.2 To manipulate the strain values of in predicting the behaviour of the flush end connection from isolated tests in term of the capacity.
- 1.3.3 To obtain the related deformation of the connection.

1.4 Scope of Studies

In this research, a scope of this study is Analytical investigation of the experimental work based on the data obtained from the strain gauge. This study focuses on a bolted flush end plate connection, by connecting the end plate at the end of trapezoidal web profile beam to the flange of an U-C section of a column. The data of strain gauge were obtained from experimental tests done by Md.Tahir *et al.* (2006). The result comparison is choosing with inclinometer experiment done by Md.Tahir *et al.* (2006).

CHAPTER 2

LITERATURE REVIEW

2.1 Beam to Column Connection

Beam-to-column connections are important elements of a steel frame and their behavior influenced its performance under the load. Beam-to-column connections are neither ideally pinned nor ideally fixed and posses a finite nonzero stiffness. However, many researchers reveal that in reality the connections behave in between the two extreme assumptions and possess some rotational stiffness.

Connection between beam and column fall into two broad categories, those intended to carry moment and those intended to carry vertical shear only. End plate connection, framed connections and seat brackets are generally designed to carry vertical shear only, as part of a simply supported design. With web end plate and web framed beam connections is particular, a theory has been expounded whereby yielding of the end plate or angles should be allowed in order to reduce the moment that is transmitted from the beam to the column [5].

Several forms of simple beam-to-column connections are illustrated in Figure

2.1.

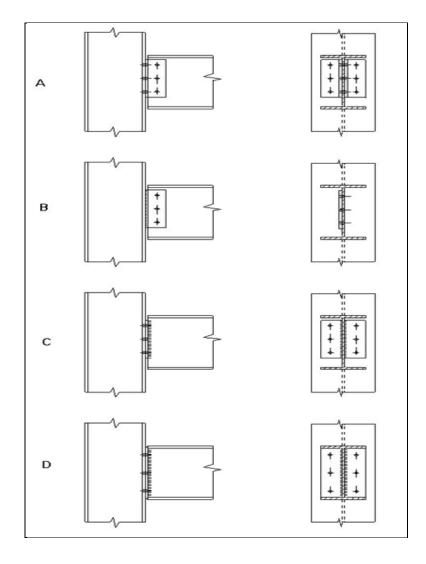


Figure 2.1: Beam to column connection, A) fully bolted, B) Finplate C) Endplate, D) Flush Endplate

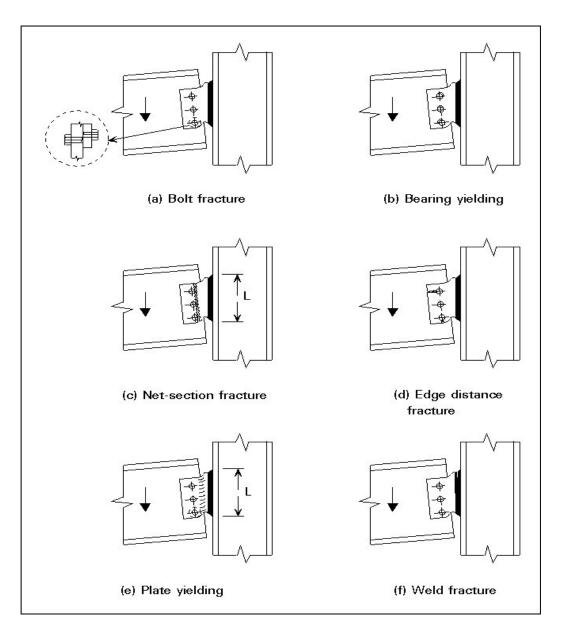


Figure 2.2: Illustrates the 6 possible failure modes for a fin plate connection

The characteristics of a connection can be best understood by considering its rotation under load. The connections rotate at an angle due to applied bending moment as shown in Figure 2.3. This connection deformation will increase the drift of the frame and causes a decrease in effective stiffness of the connected member. An increase in the frame drift will multiply the second-order (P- Δ) effects of beam-column members and thus will affect the overall stability of the frame.

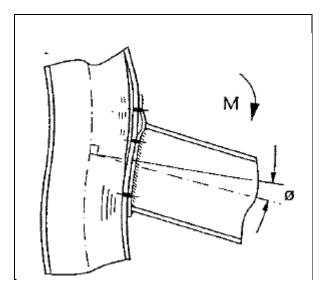


Figure 2.3: Moment-Rotation of connection [6].

2.2 Types of Connection

The stability and performance of a steel structure will be influenced by the connection between beam and column. The connections are used to transfer forces and moment due to the applied loading. Connections in steelworks are classified based on the capability of the connection in resisting moments. With this criterion, connections are divided into three types: simple connection, semi-rigid connection and rigid connection.

2.2.1 Simple Connection

Simple connections are only designed to transmit shear from the beam to the column, at some nominal eccentricity from the face of the column [7, 8, and 10]. Simple connection is providing zero rotational restraint at connections; they possess sufficiently low stiffness and are thus incapable of transmitting significant moments at ultimate limit state. Simple connection are designed can transmit end shear and axial force only. Under simple connections, there are three common types of connections that are frequently been used: web angle cleat, flexible endplate and fin plate. The simple connection is adapted to the steel structure because is easy to design and construct. Figure 2.4 show the characteristic of simple connection.

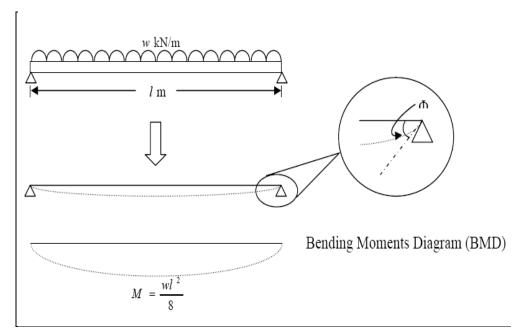


Figure 2.4: Simple connections

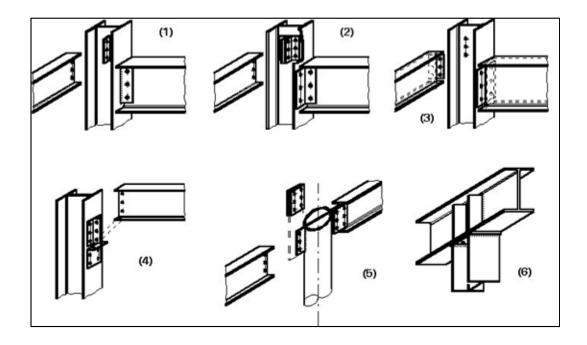


Figure 2.5: Simple beam to column connection,1) fin plates, 2) angle cleats, 3)flexible endplate welded, 4)bolted with angle cleats, 5) tube, 6) stiffness plate

Simple connection is those which comply with the following five conditions [8]:

- a) Are assumed to transfer the design shear reaction only between members.
- b) Are incapable of transmitting significant moments which might adversely affect members of the structure.
- c) Are capable of accepting the resulting rotation.
- d) Provide the directional restraint to members which have been assumed in the member design.
- e) Have sufficient robustness to satisfy the structural integrity requirement.