Author's full name : _	EHSAN	NOROOZINEJAD FARSANGI
Date of birth :	13 Sept	tember 1986
ïtle :	Seismic	Vulnerability Analysis of Various
-	Types	of Dams with Finite Element Methods
- Academic Session:	2009/20	010 - III
I declare that this thesi	s is classifie	ed as :
CONFIDEN		(Contains confidential information under the Official Sec Act 1972)*
RESTRICTED		(Contains restricted information as specified by the organization where research was done)*
OPEN ACC		I agree that my thesis to be published as online open acces (full text)
I acknowledged that l	Universiti Te	knologi Malaysia reserves the right as follows:
2. The Library of U of research only	niversiti Tek y.	of Universiti Teknologi Malaysia. Anologi Malaysia has the right to make copies for the purpos An make copies of the thesis for academic exchange.
-	·	Certificator:
SIGNATU	RE	SIGNATURE OF SUPERVISOR
V1168407	3	Prof. Dr. AZLAN ADNAN

UNIVERSITI TEKNOLOGI MALAYSIA PSZ 19:16 (Pind. 1/07)

**NOTES**: If the thesis is CONFIDENTAL or RESTRICTED, please attach with the letter from the organization with period and reasons for confidentiality or restriction.

# Fakulti Kejuruteraan Awam Universiti Teknologi Malaysia

### PENGESAHAN PENYEDIAAN SALINAN E-PROJECT REPORT

# Judul tesis : Seismic Vulnerability Analysis of Various Types of Dams with Finite Element Methods

Ijazah : Master of Engineering (Civil – Structure) Fakulti : Fakulti Kejuruteraan Awam Sesi pengajian : 2009/2010 – 3

#### Saya: EHSAN NOROOZINEJAD FARSANGI

#### (HURUF BESAR)

No Kad Pengenalan V11684073 mengaku telah menyediakan salinan e-Project report.sama seperti tesis asal yang telah diluluskan oleh panel pemeriksa dan mengikut panduan penyediaan Tesis dan Disertasi Elektronik (TDE), Sekolah Pengajian Siswazah, Universiti Teknologi Malaysia, March 2010.

1:	
2	-

Apren

(Tandatangan pelajar)

Alamat tetap:

(Tandatangan penyelia sebagai saksi)

Penyelia: Prof. Dr. AZLAN ADNAN
Fakulti : Fakulti Kejuruteraan Awam
Tarikh: 25 June 2010
Tarikh: 25 June 2010

Nota: Borang ini yang telah dilengkapi hendaklah dikemukakan kepada FKA bersama penyerahan CD. "I hereby declare that I have read this project report and in my opinion this report is sufficient in terms of scope and quality for the award of the degree of Master of Engineering (Civil – Structure)."

Signature

Name of Supervisor Date

or : Prof. Dr. AZLAN ADNAN : 25 June 2010

# SEISMIC VULNERABILITY ANALYSIS OF VARIOUS TYPES OF DAMS WITH FINITE ELEMENT METHODS

# EHSAN NOROOZINEJAD FARSANGI

A project report submitted in partial fulfillment of the requirements for the award of the degree of Master of Engineering (Civil-Structure)

> Faculty of Civil Engineering Universiti Teknologi Malaysia

> > JUNE 2010

# **Panel Members:**

1-**Dr Airil Yasreen Mohd. Yassin** B. Sc.(Civil Eng.)(Hons) Master(UTM) M.Sc (Civil Eng.)(UTM) PhD (Steel structure)(Imperial College)

# 2-Dr Roslida Abd. Samat

B.Sc.(Hons)(Long Beach), M.Sc.(Struct. Eng.)(Liverpool)

# 3-Dr. Redzuan Abdullah

B.Sc.(Civ. Eng.)(Hartford) M.Eng. (Struct.)(Cornell) Ph.D (Civil Eng.)(Virginia Tech.) I declare that this project report entitled "*Seismic Vulnerability Analysis of Various Types of Dams with Finite Element Methods*" is the result of my own research except as cited in the references. This project has not been accepted for any degree and is not concurrently submitted in candidature of any other degree.

	(iii)
Signature	
Name of Writer	: EHSAN NOROOZINEJAD FARSANGI
Date	: 25 JUNE 2010

Especially Dedicated To My Beloved Father And Mother For Their Endless Love And Support

## ACKNOWLEDGEMENTS

I would like to express my sincere appreciation to my supervisor, Professor Dr. Azlan Adnan for his encouragement, guidance, critics, friendship and help during the development of this project report.

The author wishes to thank Mr. Meldi Suhatril, for his helpful guidance.

Last but not least, the author would like to express his deepest gratitude to his parents for their understanding and love.

### ABSTARCT

In recent years, the seismic verification of structures has dramatically evolved. Malaysia is surrounded by countries such as Indonesia and Philippine that has experienced many great earthquakes; hence it would be unwise to totally ignore the effects of earthquakes on structures in Malaysia.

The purpose of this study is to investigate the vulnerability of existing Dams in Malaysia region under earthquake ground motion because In case of severe ground motions, substantial cracking is likely to develop across significant regions of the dam, and its consequences must be taken into account for a rigorous seismic evaluation. By evaluating the seismic performance of the dams we can predict dam's resistance to prevent economic damage and loss of human life in case of dam failure. In this study we present the results of the case studies of the earthquake response on various types of dams (Concrete, Rockfill and Earthfill Dams). To achieve this purpose, dams have been remodeled using SAP2000 and PLAXIS. Three different analysis methods (Free Vibration, Time History and Response Spectrum) have been implemented in this Project to study the behavior of these dams under earthquake loading, Seismic hazard was represented by the peak ground acceleration at the dam site.

After analyzing the dams in SAP2000 and PLAXIS we will compare the existing stress from the output data with dams' capacity to identify the critical parts of the dams under earthquake loading. This problem can be efficiently tackled with necessary boundary conditions with finite element technique. The dams considered in the present investigation, are analyzed by two dimensional plane strain formulations with four node quadrilateral elements in SAP2000 and 15-noded triangular elements in PLAXIS. We have done the response spectrum and linear time history analysis by using SAP2000 and Non-linear dynamic analysis by means of PLAXIS. Both softwares have included soil-structure interaction in analysis.

#### ABSTRAK

Dalam beberapa tahun terakhir, pengesahan seismik struktur telah berkembang secara dramatik. Malaysia dikelilingi oleh negara-negara seperti Indonesia dan Filipina yang telah mengalami banyak gempa bumi; maka tidak bijaksana untuk sama sekali mengabaikan kesan daripada gempa pada struktur di Malaysia.

Tujuan kajian ini adalah untuk mengkaji ketahanan empangan yang ada di Malaysia yang dipengaruhi gerakan tanah oleh gempa. Dalam kes gerakan tanah yang kuat, besar kemungkinan keretakan di kawasan penting, dan kemungkinan yang harus diambil kira untuk penilaian sismik yang ketat. Dengan menilai prestasi sismik, anggaran perlawanan rintangan untuk mengelakkan kerosakan ekonomi dan kehilangan nyawa manusia dalam hal kegagalan empangan. Dalam kajian ini, hasil kajian kes dari gempa pada pelbagai jenis empangan (konkrit, batuan dan tanah) dibentangkan. Untuk mencapai matlamat ini, empangan telah di analisa dengan menggunakan SAP2000 dan PLAXIS. Tiga kaedah analisis yang berbeza (Bebas Getaran, Perubahan Masa dan Respon Spektrum) telah dilaksanakan dalam tugas untuk mempelajari perilaku empangan, risiko gempa yang diwakili oleh percepatan tanah maksimum di lokasi empangan.

Setelah menganalisis empangan di SAP2000 dan PLAXIS perbandingan tegasan yang ada dari data keluaran dibuat dengan kapasiti empangan untuk mengenalpasti bahagian kritis dari empangan yang dibebani dengan gempa. Masalah ini dapat diatasi dengan memerhati keadaan had yang diperlukan dengan teknik unsur terhingga. Empangan-empangan yang dipertimbangkan dalam penyelidikan ini, dianalisis oleh dua formulasi dimensi iaitu satah terikan dengan empat nod dan elemen segiempat di SAP2000 dan unsur-15 segitiga nod di PLAXIS. Kami telah melakukan analisis spektrum respon dan analisi perubahan masa secara linar dengan menggunakan SAP2000 dan analisis tidak linar dinamik dengan menggunakan PLAXIS. Kedua-dua perisian telah memasukkan interaksi tanah-struktur dalam analisis.

# TABLE OF CONTENTS

CHAPTER	TITLE	PAGE
	DECLARATION	ii
	DEDICATION	iii
	ACKNOWLEDGEMENTS	iv
	ABSTRACT	v
	ABSTRAK	vi
	TABLE OF CONTENTS	vii
	LIST OF TABLES	xi
	LIST OF FIGURES	xiv
	LIST OF APPENDICES	xxiv
	LIST OF SYMBOLS	XXV
1	INTRODUCTION	1
	1.1 Introduction	1
	1.2 Problem Statement	2
	1.3 Objectives	4
	1.4 Scope of the Study	4
2	THEORETICAL BACKGROUND	6
	2.1 Classification of Dams	6
	2.1.1 Embankment Dam Types And Characteristics	6
	2.1.2 Various types of concrete dam	10
	2.2 Natural Periods Vibration of Dams	12

2.3 Damping Ratio in Dams	13
2.4 Loads on Dams	14
2.5 Hydrodynamic Pressure	15
2.6 Seismicity and seismic load effects on Dams	16
2.7 Modeling Of Seismic Response (Shaking Table)	18
2.7.1 Shake Table Experiments	18
2.8 Classification of Seismic Analysis Methods	19
2.8.1 Response Spectrum Method	19
2.8.2 Dynamic Time History Analysis (TH)	20
2.8.3 Free Vibration	21
2.9 Soil-Structure Interaction	22
2.10 SAP 2000 Program	23
2.11 PLAXIS Program	24
2.12 Vulnerability Analysis	24
2.13 Plane Strain Theory	25
2.14 Finite Element Method	26
2.15 Elements Type	27
2.16 Non-Linear Analysis	28
2.16.1 Material Non-Linearity	29
2.16.1.1 Modified Drucker-Prager Cap model	29
2.16.1.2 Mohr - Coulomb Model	30
2.16.2 Geometric Non-Linearity	34
2.17 Resonance Phenomenon in Structures (Dams)	35
2.17.1 Fast Fourier Transform (FFT)	36
LITERATURE REVIEW	38
3.1 Previous Dams Seismic Response Studies	38
3.2 Failure of Earthfill Dams due to Earthquake	45

3

3.2.1 Sliding failure	46
3.2.2 Liquefaction failure	46
3.2.3 Longitudinal cracks	47
3.2.4 Transverse cracks	48
3.2.5 Embankment And Foundation Piping	49
3.2.6 Seepage	50
3.3 Simplified Time History Analysis	50
3.4 Modal Analysis and Dam Vibration Mode Frequencies With and Without Supports	51
3.5 Modeling of Aged Concrete Dams	53
3.6 USACE Guidelines for Time History Analysis of	54
Dams	
3.6.1 Parameters in USACE method	56
3.7 Analysis of Gravity Dams and Foundation	57
3.7.1 The Massless Foundation Approximation	58
METHODOLOFY	59
4.1 Introduction and Methodology Flowchart	59
4.1.1 Penang - Ayer Itam Dam	61
4.1.2 Johor - Labong Dam	65
4.1.3 Kuala Lampur Batu Dam	69
4.1.4 Pahang - Jor Dam	74
4.1.5 Sarawak Batang Ai Dam	79
4.1.6 Sabah-Babagon Dam	83
4.1.7 Terengganu - Kenyir Dam	87
4.1.8 Perak - Chenderoh Dam	92
4.1.9 Material Properties	97
4.1.10 Material Nonlinearity (PLAXIS)	98

4

	4.1.10.1 Modified Drucker-Prager Cap Model	98
	4.1.10.2 Mohr - Coulomb model (MC)	99
	4.1.11 Time-History Analysis	101
	4.1.12 Response Spectrum Analysis	102
5	<b>RESULTS &amp; DISCUSSIONS</b>	103
	5.1 Free Vibration Analysis Results	103
	5.2 PLAXIS and SAP2000 Results	107
	5.2.1 Earthfill (Case1: Jor Dam)	109
	5.2.2 Rockfill (Case1: Batang-Ai Dam)	114
	5.2.3 Concrete (Case1: Chenderoh Dam)	119
	5.3 Summary Of The Results	124
6	CONCLUSIONS & RECOMMENDATIONS	127
	6.1 Conclusions	127
	6.2 Recommendations	128
	REFERENCES	129
	APPENDIX A: RESULTS FOR OTHER DAMS	135
	A.1 Earthfill (Case2: Ayer Itam Dam)	136
	A.2 Earthfill (Case3: Labong Dam)	141
	A.3 Earthfill (Case4: Batu Dam)	146
	A.4 Rockfill (Case2: Babagon Dam)	151
	A.5 Rockfill (Case3: Kenyir Dam)	156

# LIST OF TABLES

TABLE NO.	TITLE	PAGE
3.1	Selective Results of Dam-Foundation Analysis	57
3.2	Selective Results of Dam with Massless Foundation Analysis	58
4.1	Table of significant nodes(Ayer Itam Dam)	62
4.2	FE Software Used, Type of Elements and Total No. of Elements in Model(Ayer Itam Dam)	63
4.3	Modal Analysis Results (Ayer Itam Dam)	64
4.4	Table of significant nodes(Labong dam)	66
4.5	FE Software Used, Type of Elements and Total No. of Elements in Model(Labong dam)	67
4.6	Modal Analysis Results (Labong Dam)	68
4.7	Table of significant nodes(Batu dam)	71
4.8	FE Software Used, Type of Elements and Total No. of Elements in Model(Batu dam)	72
4.9	Modal Analysis Results (Batu dam)	73
4.10	Table of significant nodes(Jor dam)	76
4.11	FE Software Used, Type of Elements and Total No. of Elements in Model(Jor dam)	77
4.12	Modal Analysis Results (Jor dam)	78
4.13	Table of significant nodes(Batang-Ai dam)	80
4.14	FE Software Used, Type of Elements and Total No. of Elements in Model(Batang-Ai dam)	81
4.15	Modal Analysis Results (Batang-Ai dam)	82
4.16	Table of significant nodes (Babagon dam)	84
4.17	FE Software Used, Type of Elements and Total	85

4.18	Modal Analysis Results (Babagon dam)	86
4.19	Table of significant nodes(Kenyir dam)	89
4.20	FE Software Used, Type of Elements and Total No. of Elements in Model(Kenyir dam)	90
4.21	Modal Analysis Results (Kenyir dam)	91
4.22	Table of significant nodes (Chenderoh dam)	94
4.23	FE Software Used, Type of Elements and Total No. of Elements in Model(Chenderoh dam)	95
4.24	Modal Analysis Results (Chenderoh dam)	96
4.25	Material Properties	97
5.1	Natural Period of the Desired Dams	104
5.2	Verification of Natural Period for All Dams	106
5.3	The results of Linear Time History Analysis(Jor dam)	111
5.4	The results of Non-Linear Time History Analysis(Jor dam)	112
5.5	The results of Response Spectrum Analysis(Jor dam)	113
5.6	The results of Linear Time History Analysis(Batang-Ai dam)	116
5.7	The results of Non-Linear Time History Analysis(Batang-Ai dam)	117
5.8	The results of Response Spectrum Analysis(Batang-Ai dam)	118
5.9	The results of Linear Time History Analysis(Chenderoh dam)	121
5.10	The results of Non-Linear Time History Analysis(Chenderoh dam)	122
5.11	The results of Response Spectrum Analysis(Chenderoh dam)	123

5.12	Summarization of Results and Dams' Capacities	124
5.13	Results to Capacity ration for dams in percentage	125
A-1	The results of Linear Time History Analysis(Ayer Itam)	138
A-2	The results of Non-Linear Time History Analysis(Ayer Itam)	139
A-3	The results of Response Spectrum Analysis(Ayer Itam)	140
A-4	The results of Linear Time History Analysis(Labong)	143
A-5	The results of Non-Linear Time History Analysis(Labong)	144
A-6	The results of Response Spectrum Analysis(Labong)	145
A-7	The results of Linear Time History Analysis(Batu)	148
A-8	The results of Non-Linear Time History Analysis(Batu)	149
A-9	The results of Response Spectrum Analysis(Batu)	150
A-10	The results of Linear Time History Analysis(Babagon)	153
A-11	The results of Non-Linear Time History Analysis(Babagon)	154
A-12	The results of Response Spectrum Analysis(Babagon)	155
A-13	The results of Linear Time History Analysis(Kenyir)	158
A-14	The results of Non-Linear Time History Analysis(Kenyir)	159
A-15	The results of Response Spectrum Analysis(Kenyir)	160

# LIST OF FIGURES

FIGURE NO.	TITLE	PAGE
1.1	Historical earthquakes around Peninsular Malaysia ( $M\omega > 5$ )	3
1.2	Active Faults Around Peninsula Malaysia	3
2.1	Earthfills and Rockfill in dam Construction	8
2.2	Principal variants of Earthfill and Earthfill- Rockfill embankment dams(values of m are indicative only)	8
2.3	Principal variants of Rockfill embankment dams	9
	(values of m are indicative only)	
2.4	Principal variants of concrete dams (values of m and n indicative only; in (e) $R_H$ and $R_V$ generally vary over dam faces)	10
2.5	Further variants of concrete dams	12
2.6	Schematic of principal loads: gravity dam profile	15
2.7	Schematic of internal body forces: embankment dam	15
2.8	The plot of dynamic amplification equation	21
2.9	Soil-Structure Interaction Model	22
2.10	Types of quadrilateral element(Plane, General) used is SAP2000	28
2.11	Triangular element(15-Nodes, 12-Stress Points) used is PLAXIS	28
2.12	Drucker-Prager Cap model: yield surface in the $p - q$ plane	29
2.13	Schematics of a density-dependent Drucker- Prager Cap model: (a) 3D yield surfaces in principal stress space (1/4 model); (b) 2D	30

representation

2.14	Basic idea of an elastic perfectly plastic model	31
2.15	The Mohr-Coulomb yield surface in principal stress space ( $c = 0$ )	31
2.16	Definition of $E_0$ and $E_{50}$ for standard drained triaxial test results	32
2.17	Stress circles at yield; one touches Coulomb's envelope	33
3.1	Sliding failure of an earth dam	46
3.2	Liquefaction failure of an earth dam	47
3.3	Failure in terms of longitudinal cracks in an earth dams	48
3.4	Transverse cracks due differential settlements in an earth dam	49
3.5	Embankment and foundation piping in an earth dam	49
3.6	phreatic surface in earth dams	50
3.7	Upstream view of Karoon-1 dam	52
3.8	Finite Element model of Karoon-1 dam in ABAQUS	52
3.9	Geometry of dam-reservoir system	53
3.10	Models of Dam with Small and Large Foundation	57
4.1	Methodology flowchart	60
4.2	Upstream of Ayer Itam dam	61
4.3	Main Embankment Cross-Section of Ayer Itam dam	61
4.4	Plot of geometry model with significant nodes (Ayer Itam dam)	62
4.5	Meshed Structure in PLAXIS (Ayer Itam dam)	63

4.6	Meshed Structure in SAP2000(Ayer Itam dam)	63
4.7	Mode Shapes for Ayer Itam dam	64
4.8	Main Embankment Cross-Section of Labong dam	65
4.9	Plot of geometry model with significant nodes (Labong dam)	66
4.10	Meshed Structure in PLAXIS(Labong dam)	67
4.11	Meshed Structure in SAP2000(Labong dam)	67
4.12	Mode Shapes for Labong dam	68
4.13	Upstream of Batu dam	70
4.14	Plot of geometry model with significant nodes (Batu dam)	71
4.15	Meshed Structure in PLAXIS (Batu dam)	72
4.16	Meshed Structure in SAP2000 (Batu dam)	72
4.17	Mode Shapes for Batu dam	73
4.18	Upstream of Jor dam	75
4.19	Main Embankment Cross-Section of Jor dam	75
4.20	Plot of geometry model with significant nodes (Jor dam)	76
4.21	Meshed Structure in PLAXIS (Jor dam)	77
4.22	Meshed Structure in SAP2000 (Jor dam)	77
4.23	Mode Shapes for Jor dam	78
4.24	Upstream of Batang-Ai dam	79
4.25	Plot of geometry model with significant nodes (Batang-Ai dam)	80
4.26	Meshed Structure in PLAXIS (Batang-Ai dam)	81
4.27	Meshed Structure in SAP2000 (Batang-Ai dam)	81
4.28	Mode Shapes for Batang-Ai dam	82

4.29	General layout of Babagon dam	83
4.30	Plot of geometry model with significant nodes (Babagon dam)	84
4.31	Meshed Structure in PLAXIS (Babagon dam)	85
4.32	Meshed Structure in SAP2000 (Babagon dam)	85
4.33	Mode Shapes for Babagon dam	86
4.34	Downstream of Kenyir Dam	88
4.35	Powerhouse in the Downstream of Kenyir Dam	88
4.36	Plot of geometry model with significant nodes (Kenyir dam)	89
4.37	Meshed Structure in PLAXIS (Kenyir dam)	90
4.38	Meshed Structure in SAP2000 (Kenyir dam)	90
4.39	Mode Shapes for Kenyir dam	91
4.40	Upstream and downstream view of Chenderoh dam	93
4.41	General Layout of Chenderoh dam	93
4.42	Main Embankment Cross Section of Chenderoh dam	93
4.43	Plot of geometry model with significant nodes (Chenderoh dam)	94
4.44	Meshed Structure in PLAXIS(Chenderoh dam)	95
4.45	Meshed Structure in SAP2000(Chenderoh dam)	95
4.46	Mode Shapes for Chenderoh dam	96
4.47	Drucker-Prager Cap model: yield surface in the $p - q$ plane	98
4.48	Schematics of a density-dependent Drucker- Prager Cap model: (a) 3D yield surfaces in principal stress space (1/4 model); (b) 2D representation.	99
4.49	Basic idea of an elastic perfectly plastic model	100

4.50	The Mohr-Coulomb yield surface in principal stress space ( $c = 0$ )	100
4.51	Time History at Surface	101
4.52	Acceleration Response Spectrum at Surface	102
5.1	FFT graph for RapidKl Time History Dataset(SIGVIEW)	105
5.2	FFT 3D graph for RapidKl Time History Dataset(SIGVIEW)	105
5.3	Comparison The Differences Between SAP2000 Results And Theoretical Values In Terms Of Natural Period	107
5.4	Water flow path in Jor Dam's body (Shading)	109
5.5	Degree of Saturation in Jor Dam (Contour Lines)	109
5.6	Acceleration-Time diagram for the dam's crest (Jor dam)	110
5.7	Displacement-Time diagram for the dam's crest (Jor dam)	110
5.8	Normal Stress diagram for Linear Time History Analysis (Jor dam)	111
5.9	Shear Stress diagram for Linear Time History Analysis (Jor dam)	111
5.10	Normal Stress diagram for Non-Linear Time History Analysis (Jor dam)	112
5.11	Shear Stress diagram for Non-Linear Time History Analysis (Jor dam)	112
5.12	Normal Stress diagram for Response Spectrum Analysis (Jor dam)	113
5.13	Shear Stress diagram for Response Spectrum Analysis (Jor dam)	113
5.14	Water flow path in Batang-Ai Dam's body (Shading)	114
5.15	Degree of Saturation in Batang-Ai Dam (Contour	114

Lines)

5.16	Acceleration-Time diagram for the dam's crest(Batang-Ai dam)	115
5.17	Displacement-Time diagram for the dam's crest(Batang-Ai dam)	115
5.18	Normal Stress diagram for Linear Time History Analysis(Batang-Ai dam)	116
5.19	Shear Stress diagram for Linear Time History Analysis(Batang-Ai dam)	116
5.20	Normal Stress diagram for Non-Linear Time History Analysis(Batang-Ai dam)	117
5.21	Shear Stress diagram for Non-Linear Time History Analysis(Batang-Ai dam)	117
5.22	Normal Stress diagram for Response Spectrum Analysis(Batang-Ai dam)	118
5.23	Shear Stress diagram for Response Spectrum Analysis(Batang-Ai dam)	118
5.24	Water flow path in Chenderoh Dam's body (Shading)	119
5.25	Degree of Saturation in Chenderoh dam(Contour Lines)	119
5.26	Acceleration-Time diagram for the dam's crest(Chenderoh dam)	120
5.27	Displacement-Time diagram for the dam's crest(Chenderoh dam)	120
5.28	Normal Stress diagram for Linear Time History Analysis(Chenderoh dam)	121
5.29	Shear Stress diagram for Linear Time History Analysis(Chenderoh dam)	121
5.30	Normal Stress diagram for Non-Linear Time History Analysis(Chenderoh dam)	122
5.31	Shear Stress diagram for Non-Linear Time History Analysis(Chenderoh dam)	122

5.32	Normal Stress diagram for Response Spectrum Analysis(Chenderoh dam)	123
5.33	Shear Stress diagram for Response Spectrum Analysis(Chenderoh dam)	123
5.34	Comparison Of Results From Different Methods	125
5.35	Comparison of Displacement for Earthfill dams	126
5.36	Comparison of Displacement for Rockfill dams	126
A-1	Water flow path in Ayer Itam Dam's body (Shading)	136
A-2	Degree of Saturation in Ayer Itam dam (Contour Lines)	136
A-3	Acceleration-Time diagram for the dam's crest(Ayer Itam)	137
A-4	Displacement-Time diagram for the dam's crest(Ayer Itam)	137
A-5	Normal Stress diagram for Linear Time History Analysis(Ayer Itam)	138
A-6	Shear Stress diagram for Linear Time History Analysis(Ayer Itam)	138
A-7	Normal Stress diagram for Non-Linear Time History Analysis(Ayer Itam)	139
A-8	Shear Stress diagram for Non-Linear Time History Analysis(Ayer Itam)	139
A-9	Normal Stress diagram for Response Spectrum Analysis(Ayer Itam)	140
A-10	Shear Stress diagram for Response Spectrum Analysis(Ayer Itam)	140
A-11	Water flow path in Labong Dam's body (Shading)	141
A-12	Degree of Saturation in Labong dam (Contour Lines)	141
A-13	Acceleration-Time diagram for the dam's	142

crest(Labong)

A-14	Displacement-Time diagram for the dam's crest(Labong)	142
A-15	Normal Stress diagram for Linear Time History Analysis(Labong)	143
A-16	Shear Stress diagram for Linear Time History Analysis(Labong)	143
A-17	Normal Stress diagram for Non-Linear Time History Analysis(Labong)	144
A-18	Shear Stress diagram for Non-Linear Time History Analysis(Labong)	144
A-19	Normal Stress diagram for Response Spectrum Analysis(Labong)	145
A-20	Shear Stress diagram for Response Spectrum Analysis(Labong)	145
A-21	Water flow path in Batu Dam's body (Shading)	146
A-22	Degree of Saturation in Batu dam (Contour Lines)	146
A-23	Acceleration-Time diagram for the dam's crest(Batu)	147
A-24	Displacement-Time diagram for the dam's crest(Batu)	147
A-25	Normal Stress diagram for Linear Time History Analysis(Batu)	148
A-26	Shear Stress diagram for Linear Time History Analysis(Batu)	148
A-27	Normal Stress diagram for Non-Linear Time History Analysis(Batu)	149
A-28	Shear Stress diagram for Non-Linear Time History Analysis(Batu)	149
A-29	Normal Stress diagram for Response Spectrum Analysis(Batu)	150

A-30	Shear Stress diagram for Response Spectrum Analysis(Batu)	150
A-31	Water flow path in Babagon Dam's body (Shading)	151
A-32	Degree of Saturation in Babagon dam (Contour Lines)	151
A-33	Acceleration-Time diagram for the dam's crest (Babagon)	152
A-34	Displacement-Time diagram for the dam's crest(Babagon)	152
A-35	Normal Stress diagram for Linear Time History Analysis(Babagon)	153
A-36	Shear Stress diagram for Linear Time History Analysis(Babagon)	153
A-37	Normal Stress diagram for Non-Linear Time History Analysis (Babagon)	154
A-38	Shear Stress diagram for Non-Linear Time History Analysis(Babagon)	154
A-39	Normal Stress diagram for Response Spectrum Analysis(Babagon)	155
A-40	Shear Stress diagram for Response Spectrum Analysis(Babagon)	155
A-41	Water flow path in Kenyir Dam's body (Shading)	156
A-42	Degree of Saturation in Kenyir dam(Contour Lines)	156
A-43	Acceleration-Time diagram for the dam's crest(Kenyir)	157
A-44	Displacement-Time diagram for the dam's crest(Kenyir)	157
A-45	Normal Stress diagram for Linear Time History Analysis(Kenyir)	158
A-46	Shear Stress diagram for Linear Time History	158

Analysis(Kenyir)

A-47	Normal Stress diagram for Non-Linear Time History Analysis(Kenyir)	159
A-48	Shear Stress diagram for Non-Linear Time History Analysis(Kenyir)	159
A-49	Normal Stress diagram for Response Spectrum Analysis(Kenyir)	160
A-50	Shear Stress diagram for Response Spectrum Analysis(Kenyir)	160

# LIST OF APPENDICES

APPENDIX	TITLE	PAGE
А	Results For Other Dams(SAP2000 and PLAXIS)	135

# LIST OF SYMBOLS

Н	dam height
В	dam width
γ	density
E	Young's modulus
E <sub>m</sub>	degraded elastic modulus
E <sub>0</sub>	original modulus
g	gravity
р	hydrodynamic pressure
m	equivalent mass of water storage
С	ratio between seismic acceleration and gravitational acceleration
W	density of water (kg/m <sup>3</sup> )
у	distance of cross section from dam crest (m)
u	unknown relative displacements vectors
Ú	unknown relative velocities vectors
ü	unknown relative accelerations vectors
М	mass matrix of the soil-structure system
С	damping matrix
D	dynamic amplification factors
β	frequency ratio
ξ	damping ratio
ω	frequency of earthquake excitation
ω	frequency of structure
ω <sub>i</sub>	angular frequencies of vibration modes

- $\sigma_i$  Stress in *i* direction
- $v, \mu$  Poisson's ratio
- $\phi$  Friction angle
- $\psi$  Dilatancy angle
- $\varepsilon_i$  Strain in *i* direction
- $\phi \qquad \qquad \text{total porosity of concrete} \\$

## **CHAPTER 1**

### **INTRODUCTION**

# 1.1 Introduction

An earthquake is produced by the sudden rupture or slip of a geological fault. Faults occur at the intersection of two segments of the earth's crust. Peninsula Malaysia lies in the Eurasian Plate and also within the Indian-Australian Plate. Geologically, small faults also exist in East Malaysia. Records have shown that we do sometimes experiences some off-set tremors originating from the Indonesian zone. Thus there is a need for some seismic checking to be incorporated in the design process so that the structures would be resistant to earthquake.

In the recent years, the issues of seismic safety of dams have become a major concern in the planning and designing of new dams proposed to be built and for safety evaluation of existing dams in seismic regions. Prediction of the performance of the dams during earthquakes is one of the most challenging and complex problems found in the field of structural dynamics.

Water with good quality and in sufficient quantity is a basic requirement for humanity. Reservoirs and dams that create those reservoirs provide a means to balance the fluctuation of natural water flow. Multipurpose reservoirs can serve for drinking water, irrigation in agriculture, production of clean and renewable energy, recreation and flood protection. So, the dams and study of theirs stability under variable loading especially earthquake forces play a vital role in the infrastructure of many states for the provision of water resource and saving money.

Finite Element Method (FFM) is a numerical method that can be used to solve different kinds of engineering problems in the stable, transient, linear or nonlinear cases (*Bathe, 1996*). Among finite element method software's, SAP2000 and PLAXIS are known as the most precise and practicable softwares in industry and university researches. They are used for dynamic analysis such as earthquake and water wave loading on structures.

#### **1.2 Problem Statement**

There are many reasons for Malaysia to worry about earthquake. New revelations indicate Malaysia is moving closer towards the rumble zone. Malaysia is inching closer to rumble zones and will not be immune to earthquake forever. Year after year, neighboring tectonic plates inch towards from all directions and on mounting because the Australian, Eurasian and Philippine plate around Malaysia are moving . *Azlan (2005)* stated that Peninsular Malaysia does lie on faults but have been known to be non-active faults. Malaysia is located in low seismic activity area but the active earthquake fault line through the center of Sumatera just lies 350 km from peninsular.

Sabah, which experienced the highest earthquake magnitude recoded with 4.8 Richter in the last century occurred about 90 KM from Miri, Sarawak have cause several building crack. The plates are moving closer toward and shift a few centimeters was recorded after the incidents have been reported (*Hendriyawan et. al, 2003*). Thus, this study will demonstrate the behavior of Dams under earthquake effect. The Dams will be modeled and analyzed with SAP 2000 and PLAXIS to determine their seismic vulnerabilities.

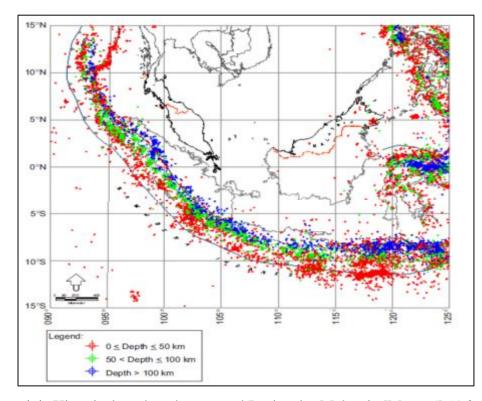


Figure 1.1: Historical earthquakes around Peninsular Malaysia [M $\omega > 5$ ] (Adnan. A. 2005)

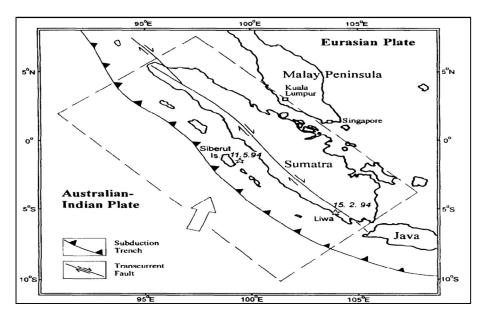


Figure 1.2: Active Faults Around Peninsula Malaysia (Sun. J. et. al 1995)

### 1.3 Objectives

The objectives of this project are:

- To remodel the Dams using finite element modeling with SAP2000 and PLAXIS.
- To determine the vulnerability of existing dams in Malaysia under earthquake load
- To determine the earthquake design criteria for new dams located in Malaysia region

- To study the performance of existing Dams while seismic activity occurs using time history, response spectrum and free vibration analysis.

### 1.4 Scope Of Study

1. Various dams from different locations have been identified for the finite element analysis. The dams are as follows:

### **Rockfill Dams:**

- Sarawak- Batang Ai Dam
- Sabah-Babagon Dam
- Terengganu-Kenyir Dam

### **Earthfill Dams:**

- Kuala Lampur- Batu Dam
- Penang- Ayer Itam Dam
- Johor- Labong Dam
- Pahang- Jor Dam

## **Concrete Dam:**

• Perak- Chenderoh Dam

2. Seismic vulnerability analysis will be performed on the identified Dams.

3. Modeling the Dams using plane strain modeling.

4. Model the dams only using 2 dimensional views.

5-Linear Response spectrum and Time History by SAP2000 and Non-Linear Time History analysis by means of PLAXIS.